STRUCTURAL CALCULATIONS

Project:

Garcia Residence 4327 Forest Avenue Southeast Mercer Island, Washington 98040

Architect:

Capsule 2366 Eastlake Avenue East, #403 Seattle, Washington 98103

Structural Engineer:

Harriott Valentine Engineers, Inc. 1932 First Avenue, Suite 720 Seattle, Washington 98101 tel. 206-624-4760



Harriott Valentine Engineers Inc.

SECTION 1: GENERAL

Harriott Valentine Engineers Inc.

CRITERIA

Gravity

Deck	dead	2x decking 2x10 @ 16"oc miscellaneous	4.3 2.8 1.9 21% 9.0 psf	live deck	60.0 psf
	total	dead + live	69.0 psf		
Lateral					
Wind		wind importance factor basic wind speed wind exposure	1.0 97 mph B		
Seismic		seismic importance factor latitude longitude mapped spectral response accel. at short periods (Ss)	1.0 47.568 ° -122.233 ° 1.430 g	(from SEAOC)	
		seismic design category response modification factor (R)	D 1.5		

USGS web services were down for some period of time and as a result this tool wasn't operational, resulting in *timeout* error. USGS web services are now operational so this tool should work as expected.

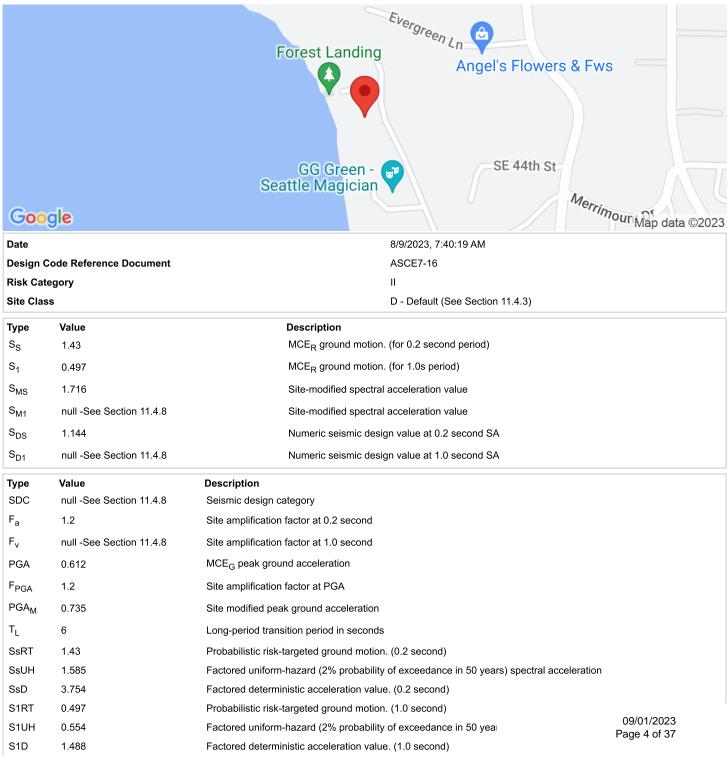




Garcia Residence

4327 Forest Ave SE, Mercer Island, WA 98040, USA

Latitude, Longitude: 47.5679413, -122.2327234



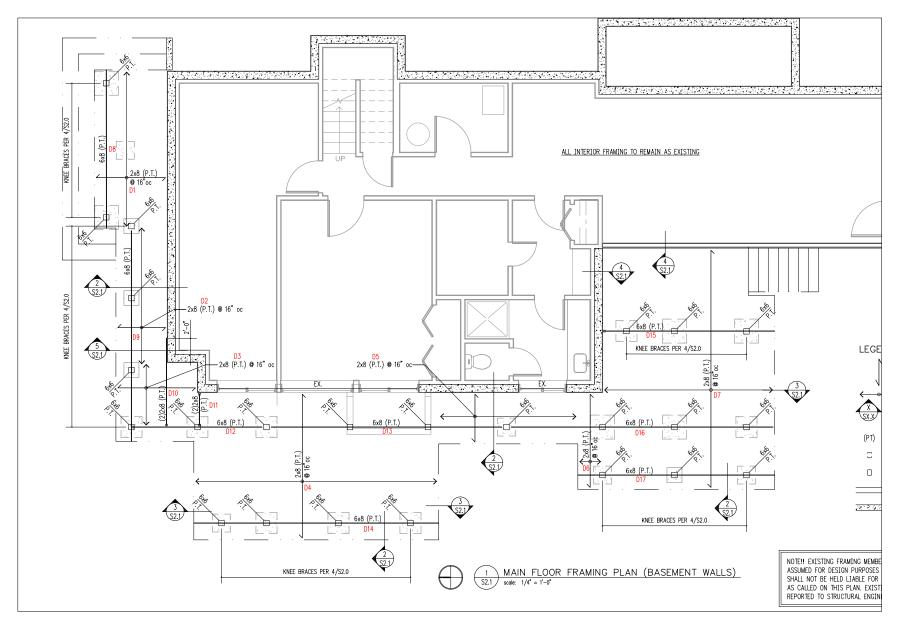
https://www.seismicmaps.org

8/9/23, 7:40 AM U.S. Seismic Design Maps Туре Value Description PGAd 1.273 Factored deterministic acceleration value. (Peak Ground Acceleration) PGA_{UH} 0.612 Uniform-hazard (2% probability of exceedance in 50 years) Peak Ground Acceleration C_{RS} 0.902 Mapped value of the risk coefficient at short periods C_{R1} 0.897 Mapped value of the risk coefficient at a period of 1 s \mathbf{C}_{V} 1.386 Vertical coefficient

Harriott Valentine Engineers Inc.

SECTION 2: FRAMING

DECK MEMBER ID





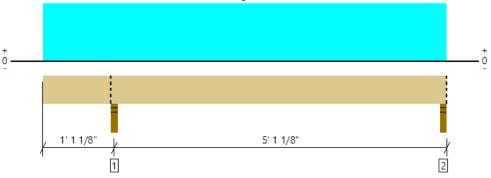
Deck							
Member Name	Results	Current Solution	Comments				
D1	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D2	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D3	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D4	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D5	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D6	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D7	Passed	1 piece(s) 2 x 8 HF No.2 @ 16" OC					
D8	Passed	1 piece(s) 6 x 8 DF No.1					
D9	Passed	1 piece(s) 6 x 8 DF No.1					
D10	Passed	2 piece(s) 2 x 8 HF No.2					
D11	Passed	2 piece(s) 2 x 8 HF No.2					
D12	Passed	1 piece(s) 6 x 8 DF No.1					
D13	Passed	1 piece(s) 6 x 8 DF No.1					
D14	Passed	1 piece(s) 6 x 8 DF No.1					
D15	Passed	1 piece(s) 6 x 8 DF No.1					
D16	Passed	1 piece(s) 6 x 8 DF No.1					
D17	Passed	1 piece(s) 6 x 8 DF No.1					





Deck, D1 1 piece(s) 2 x 8 HF No.2 @ 16" OC

Overall Length: 6' 2 1/4"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	338 @ 1' 1 1/8"	2126 (3.50")	Passed (16%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	169 @ 1' 10 1/8"	1088	Passed (16%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	276 @ 3' 6 7/8"	1284	Passed (21%)	1.00	1.0 D + 1.0 L (Alt Spans)
Live Load Defl. (in)	0.017 @ 3' 6 11/16"	0.164	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.019 @ 3' 6 3/4"	0.246	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro [™] Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

PASSED

Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (2L/360) and TL (2L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

· No composite action between deck and joist was considered in analysis.

	Bearing Length			Loads	to Supports		
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Stud wall - HF	3.50"	3.50"	1.50"	44	294	338	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	30	210/-10	241	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' 2" o/c	
Bottom Edge (Lu)	6' 2" o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 6' 2 1/4"	16"	9.0	60.0	Deck

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

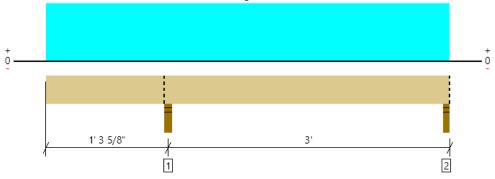
ForteWEB Software Operator Jacob Wachtendonk Harriott Valentine Engineers (425) 281-0788 jwachtendonk@harriottvalentine.com





Deck, D2 1 piece(s) 2 x 8 HF No.2 @ 16" OC

Overall Length: 4' 3 5/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	278 @ 1' 3 5/8"	2126 (3.50")	Passed (13%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	89 @ 2' 5/8"	1088	Passed (8%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	87 @ 2' 9 1/16"	1284	Passed (7%)	1.00	1.0 D + 1.0 L (Alt Spans)
Live Load Defl. (in)	0.002 @ 2' 8 5/8"	0.094	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.003 @ 0	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro [™] Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

PASSED

Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (2L/360) and TL (0.2").

• Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

· No composite action between deck and joist was considered in analysis.

	Bearing Length			Loads	to Supports		
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Stud wall - HF	3.50"	3.50"	1.50"	36	241	278	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	15	127/-24	142/-9	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 4" o/c	
Bottom Edge (Lu)	4' 4" o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 4' 3 5/8"	16"	9.0	60.0	Deck

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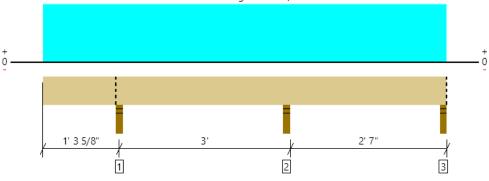
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Deck, D3 1 piece(s) 2 x 8 HF No.2 @ 16" OC

Overall Length: 6' 10 5/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	307 @ 4' 1 7/8"	2126 (3.50")	Passed (14%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	88 @ 3' 4 7/8"	1088	Passed (8%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-82 @ 4' 1 7/8"	1284	Passed (6%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.003 @ 0	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.004 @ 0	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro™ Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (0.2") and TL (0.2").

• Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

• No composite action between deck and joist was considered in analysis.

	Bearing Length		Loads to Supports (lbs)					
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories	
1 - Stud wall - HF	3.50"	3.50"	1.50"	33	233	267	Blocking	
2 - Stud wall - SPF	3.50"	3.50"	1.50"	35	271	307	None	
3 - Stud wall - HF	3.00"	3.00"	1.50"	14	111/-17	125/-3	Blocking	
• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.								

Lateral Bracing Bracing Intervals Comments

	g		
	Top Edge (Lu)	6' 11" o/c	
ĺ	Bottom Edge (Lu)	6' 11" o/c	
	Maximum allowable brasing inter	ala based on applied lead	

Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 6' 10 5/8"	16"	9.0	60.0	Deck

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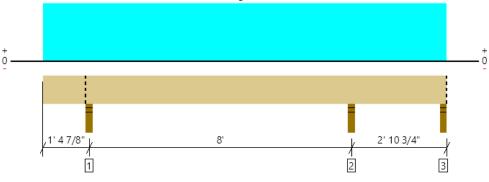
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Deck, D4 1 piece(s) 2 x 8 HF No.2 @ 16" OC

Overall Length: 12' 3 5/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	770 @ 9' 4 7/8"	2126 (3.50")	Passed (36%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	368 @ 8' 7 7/8"	1088	Passed (34%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-566 @ 9' 4 7/8"	1284	Passed (44%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.067 @ 5' 5/8"	0.267	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.075 @ 5' 5/8"	0.400	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro [™] Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

PASSED

Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (2L/360) and TL (2L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

· No composite action between deck and joist was considered in analysis.

	Bearing Length		Loads to Supports (lbs)					
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories	
1 - Stud wall - HF	3.50"	3.50"	1.50"	58	386	444	Blocking	
2 - Stud wall - SPF	3.50"	3.50"	1.50"	97	673	770	None	
3 - Stud wall - HF	3.00"	3.00"	1.50"	-7	126/-175	119/-182	Blocking	
• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.								

Lateral Bracing	Bracing Intervals	Comments				
Top Edge (Lu)	12' 4" o/c					
Bottom Edge (Lu)	12' 4" o/c					

Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 12' 3 5/8"	16"	9.0	60.0	Deck

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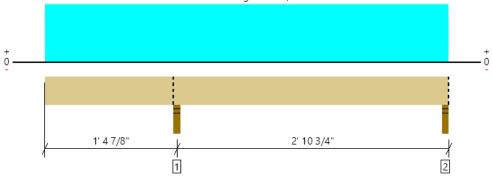
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Deck, D5 1 piece(s) 2 x 8 HF No.2 @ 16" OC

Overall Length: 4' 3 5/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	288 @ 1' 4 7/8"	2126 (3.50")	Passed (14%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	90 @ 2' 1 7/8"	1088	Passed (8%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	-91 @ 1' 4 7/8"	1284	Passed (7%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.004 @ 0	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.004 @ 0	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro [™] Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

PASSED

• Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (0.2") and TL (0.2").

• Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

· No composite action between deck and joist was considered in analysis.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Stud wall - HF	3.50"	3.50"	1.50"	38	251	288	Blocking
2 - Stud wall - HF	3.00"	3.00"	1.50"	14	123/-29	137/-15	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 4" o/c	
Bottom Edge (Lu)	4' 4" o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 4' 3 5/8"	16"	9.0	60.0	Deck

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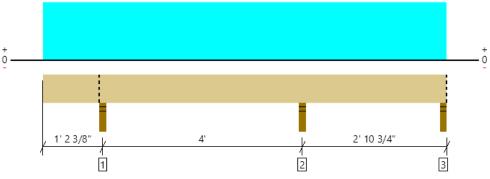
ForteWEB Software Operator Jacob Wachtendonk Harriott Valentine Engineers (425) 281-0788 jwachtendonk@harriottvalentine.com





Deck, D6 1 piece(s) 2 x 8 HF No.2 @ 16" OC

Overall Length: 8' 1 1/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	395 @ 5' 2 3/8"	2126 (3.50")	Passed (19%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	148 @ 4' 5 3/8"	1088	Passed (14%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-142 @ 5' 2 3/8"	1284	Passed (11%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.005 @ 3' 1 3/16"	0.133	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.005 @ 3' 1 3/16"	0.200	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro [™] Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

Deflection criteria: LL (L/360) and TL (L/240).

Overhang deflection criteria: LL (2L/360) and TL (0.2").

Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

· No composite action between deck and joist was considered in analysis.

	Bearing Length		Loads to Supports (lbs)					
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories	
1 - Stud wall - HF	3.50"	3.50"	1.50"	36	251	287	Blocking	
2 - Stud wall - SPF	3.50"	3.50"	1.50"	48	346	395	None	
3 - Stud wall - HF	3.00"	3.00"	1.50"	12	118/-35	130/-22	Blocking	
• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.								

 Lateral Bracing
 Bracing Intervals
 Comments

 Top Edge (Lu)
 8' 1" o/c
 Image: Comment set of the set of the

Top Euge (Eu)	010/0	
Bottom Edge (Lu)	8' 1" o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 8' 1 1/8"	16"	9.0	60.0	Deck

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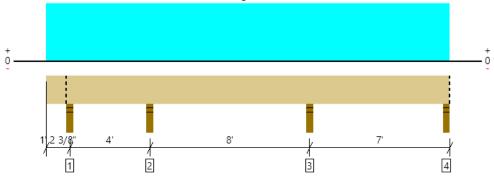




Deck, D7 1 piece(s) 2 x 8 HF No.2 @ 16" OC

PASSED

Overall Length: 20' 2 3/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	798 @ 13' 2 3/8"	2126 (3.50")	Passed (38%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	332 @ 12' 5 3/8"	1088	Passed (31%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-563 @ 13' 2 3/8"	1284	Passed (44%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.054 @ 9' 3 3/16"	0.267	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.058 @ 9' 2 7/8"	0.400	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
TJ-Pro [™] Rating	N/A	N/A	N/A		N/A

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

Deflection criteria: LL (L/360) and TL (L/240).

Overhang deflection criteria: LL (2L/360) and TL (2L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

 \bullet A 15% increase in the moment capacity has been added to account for repetitive member usage.

Applicable calculations are based on NDS.

· No composite action between deck and joist was considered in analysis.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Stud wall - HF	3.50"	3.50"	1.50"	29	278	307	Blocking
2 - Stud wall - SPF	3.50"	3.50"	1.50"	78	601	679	None
3 - Stud wall - SPF	3.50"	3.50"	1.50"	103	695	798	None
4 - Stud wall - HF	3.00"	3.00"	1.50"	33	254/-37	287/-5	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments				
Top Edge (Lu)	20' 2" o/c					
Bottom Edge (Lu)	14' 10" o/c					
Maximum allowable bracing intervals based on applied load.						

			Dead	Floor Live	
Vertical Load	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 20' 2 3/8"	16"	9.0	60.0	Deck

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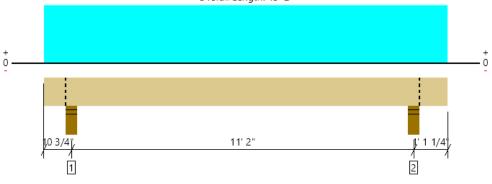
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Deck, D8 1 piece(s) 6 x 8 DF No.1

Overall Length: 13' 2"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1778 @ 12' 3/4" 12856		Passed (14%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	1261 @ 11' 2 1/2"	4675	Passed (27%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	4092 @ 6' 5 11/16"	5156	Passed (79%)	1.00	1.0 D + 1.0 L (Alt Spans)
Live Load Defl. (in)	0.249 @ 6' 5 3/4"	0.372	Passed (L/537)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.297 @ 6' 5 3/4"	0.558	Passed (L/452)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

Overhang deflection criteria: LL (2L/360) and TL (2L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

• Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Stud wall - SPF	5.50"	5.50"	1.50"	281	1437	1717	Blocking
2 - Stud wall - SPF	5.50"	5.50"	1.50"	291	1487	1778	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments			
Top Edge (Lu)	13' 2" o/c				
Bottom Edge (Lu)	13' 2" o/c				
Maximum allowable bracing intervals based on applied load					

Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	0 to 13' 2"	N/A	10.4		
1 - Uniform (PLF)	0 to 13' 2" (Top)	N/A	33.0		Linked from: D1, Support 1

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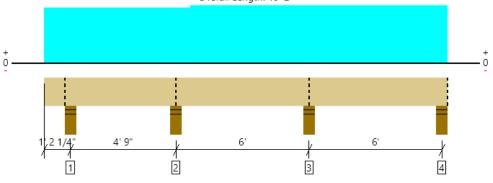
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Overall Length: 18' 2"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1544 @ 11' 11 1/4"	12856 (5.50")	Passed (12%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	607 @ 12' 9 1/2"	4675	Passed (13%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-886 @ 11' 11 1/4"	5156	Passed (17%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.012 @ 15' 7/16"	0.197	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.013 @ 15' 13/16"	0.295	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

PASSED

• Deflection criteria: LL (L/360) and TL (L/240).

Overhang deflection criteria: LL (2L/360) and TL (2L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

• Applicable calculations are based on NDS.

	Bearing Length		Loads to Supports (lbs)				
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Stud wall - SPF	5.50"	5.50"	1.50"	111	630	741	Blocking
2 - Stud wall - SPF	5.50"	5.50"	1.50"	201	1136	1337	Blocking
3 - Stud wall - SPF	5.50"	5.50"	1.50"	255	1289	1544	Blocking
4 - Stud wall - SPF	5.50"	5.50"	1.50"	99	532/-55	631	Blocking
 Blocking Panels are assumed to carry no load 	s applied dire	ctly above the	m and the ful	l load is appli	ed to the mer	nber being d	esigned.

 Lateral Bracing
 Bracing Intervals
 Comments

 Top Edge (Lu)
 18' 2" o/c
 18' 2" o/c

 Bottom Edge (Lu)
 18' 2" o/c
 18' 2" o/c

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 18' 2"	N/A	10.4		
1 - Uniform (PLF)	6' 7" to 18' 2" (Top)	N/A	27.0	180.8	Linked from: D2, Support 1
2 - Uniform (PLF)	0 to 6' 7" (Top)	N/A	24.8	174.8	Linked from: D3, Support 1

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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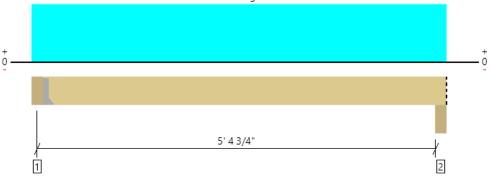
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Deck, D10 2 piece(s) 2 x 8 HF No.2





All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	622 @ 5 1/2"	1823 (1.50")	Passed (34%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	480 @ 1' 3/4"	2175	Passed (22%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	823 @ 3' 1 1/4"	2234	Passed (37%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.029 @ 3' 1 1/4"	0.176	Passed (L/999+)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.033 @ 3' 1 1/4"	0.265	Passed (L/999+)		1.0 D + 1.0 L (All Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Hanger on 7 1/4" SPF beam	5.50"	Hanger ¹	1.50"	96	631	727	See note 1
2 - Ledger - SPF	5.50"	5.50"	1.50"	95	606	700	Blocking

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

• At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger

• ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments			
Top Edge (Lu)	5' 8" o/c				
Bottom Edge (Lu)	5' 8" o/c				

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie								
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories		
1 - Face Mount Hanger	LUS26-2	2.00"	N/A	4-10dx1.5	4-10d			
- Defer to manufacturer notes and instructi	and for proper installation and use	of all compositors						

Refer to manufacturer notes and instructions for proper installation and use of all connectors.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	5 1/2" to 6' 1"	N/A	5.5		
1 - Uniform (PLF)	0 to 6' 1" (Top)	N/A	26.3	203.3	Linked from: D3, Support 2

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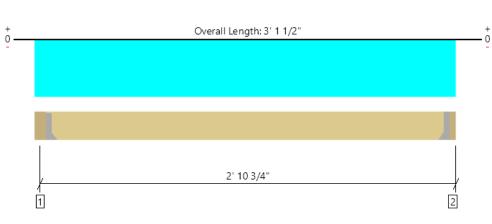
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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Deck, D11 2 piece(s) 2 x 8 HF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern) [Group]
Member Reaction (lbs)	120 @ 5 1/2"	1823 (1.50")	Passed (7%)		1.0 D + 1.0 L (All Spans) [1]
Shear (lbs)	60 @ 1' 3/4"	2175	Passed (3%)	1.00	1.0 D + 1.0 L (All Spans) [1]
Moment (Ft-lbs)	72 @ 1' 8"	2234	Passed (3%)	1.00	1.0 D + 1.0 L (All Spans) [1]
Live Load Defl. (in)	0.001 @ 1' 8"	0.081	Passed (L/999+)		1.0 D + 1.0 L (All Spans) [1]
Total Load Defl. (in)	0.001 @ 1' 8"	0.121	Passed (L/999+)		1.0 D + 1.0 L (All Spans) [1]

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

Applicable calculations are based on NDS.

	Bearing Length			Loads	to Supports		
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Hanger on 7 1/4" SPF beam	5.50"	Hanger ¹	1.50"	24	139/-21	163	See note 1
2 - Hanger on 7 1/4" SPF Ledger	3.00"	Hanger ¹	1.50"	22	121/-19	143	See note 1
• At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger							

1 See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments			
Top Edge (Lu)	2' 5" o/c				
Bottom Edge (Lu)	2' 5" o/c				
Maximum allowable bracing intervals based on applied load					

Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie							
Model	I Seat Length Top Fasteners Face Fas		Face Fasteners	Member Fasteners	Accessories		
LUS26-2	2.00"	N/A	4-10dx1.5	3-10d			
LUS26-2	2.00"	N/A	4-10dx1.5	3-10d			
	Model LUS26-2	Model Seat Length LUS26-2 2.00"	Model Seat Length Top Fasteners LUS26-2 2.00" N/A	Model Seat Length Top Fasteners Face Fasteners LUS26-2 2.00" N/A 4-10dx1.5	Model Seat Length Top Fasteners Face Fasteners Member Fasteners LUS26-2 2.00" N/A 4-10dx1.5 3-10d		

Refer to manufacturer notes and instructions for proper installation and use of all connectors.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	5 1/2" to 2' 10 1/2"	N/A	5.5		
1 - Uniform (PLF)	0 to 3' 1 1/2" (Top)	N/A	10.5	83.3/-12.8	Linked from: D3, Support 3

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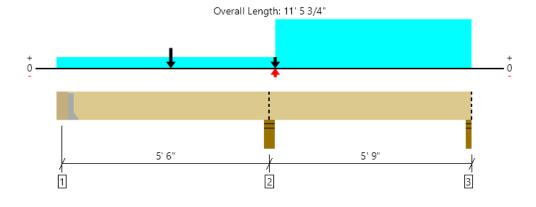
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Deck, D12 1 piece(s) 6 x 8 DF No.1



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern) [Group]
Member Reaction (lbs)	3151 @ 5' 8 3/4"	12856 (5.50")	Passed (25%)		1.0 D + 1.0 L (All Spans) [1]
Shear (lbs)	1476 @ 6' 7"	4675	Passed (32%)	1.00	1.0 D + 1.0 L (All Spans) [1]
Moment (Ft-lbs)	-1794 @ 5' 8 3/4"	5156	Passed (35%)	1.00	1.0 D + 1.0 L (All Spans) [1]
Live Load Defl. (in)	0.026 @ 8' 8 5/8"	0.188	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans) [1]
Total Load Defl. (in)	0.029 @ 8' 8 13/16"	0.282	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans) [1]

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

• Applicable calculations are based on NDS.

	Bearing Length			Loads	to Supports		
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Hanger on 7 1/2" SPF beam	5.50"	Hanger ¹	1.50"	77	568/-145	645/-67	See note 1
2 - Stud wall - SPF	5.50"	5.50"	1.50"	468	2683	3151	Blocking
3 - Stud wall - SPF	2.75"	2.75"	1.50"	197	1293/-87	1490	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

• At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger

• ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments			
Top Edge (Lu)	11' o/c				
Bottom Edge (Lu)	11' o/c				
•Maximum allowable bracing intervals based on applied load.					

based on applied loa

Connector: Simpson Strong-Tie

Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories	
1 - Face Mount Hanger	U66	2.00"	N/A	8-10dx1.5	4-10d		

• Refer to manufacturer notes and instructions for proper installation and use of all connectors.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	5 1/2" to 11' 5 3/4"	N/A	10.4		
1 - Uniform (PSF)	0 to 5' 10 3/4" (Top)	1' 10 3/4"	9.0	60.0	Default Load
2 - Uniform (PLF)	5' 10 3/4" to 11' 5 3/4" (Top)	N/A	72.8	504.8	Linked from: D4, Support 2
3 - Point (lb)	3' 1 3/4" (Front)	N/A	96	631	Linked from: D10, Support 1
4 - Point (lb)	5' 10 3/4" (Front)	N/A	24	139/-21	Linked from: D11, Support 1

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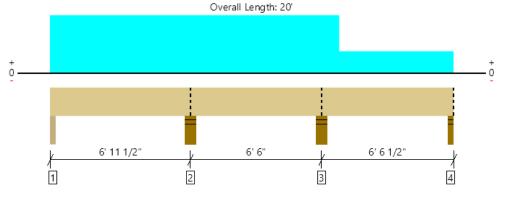
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Overall Learnith, 20



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4706 @ 6' 11 1/2"	12856 (5.50")	Passed (37%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	1960 @ 6' 1 1/4"	4675	Passed (42%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-3065 @ 6' 11 1/2"	5156	Passed (59%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.057 @ 3' 4"	0.228	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.064 @ 3' 3 3/4"	0.343	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

Allowed moment does not reflect the adjustment for the beam stability factor.

Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Beam - SPF	2.75"	2.75"	1.50"	234	1566/-151	1800	None
2 - Stud wall - SPF	5.50"	5.50"	2.01"	644	4063	4706	Blocking
3 - Stud wall - SPF	5.50"	5.50"	1.50"	439	2877	3316	Blocking
4 - Stud wall - SPF	2.75"	2.75"	1.50"	97	619/-168		Blocking

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	20' o/c	
Bottom Edge (Lu)	20' o/c	

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 20'	N/A	10.4		
1 - Uniform (PLF)	0 to 14' 4" (Top)	N/A	72.8	504.8	Linked from: D4, Support 2
2 - Uniform (PLF)	14' 4" to 20' (Top)	N/A	28.5	188.3	Linked from: D5, Support 1

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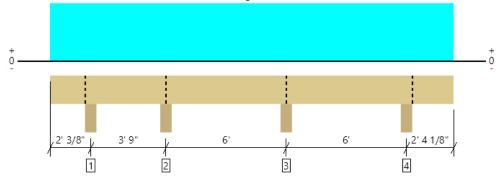
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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Overall Length: 20' 1 1/2"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4175 @ 11' 9 3/8"	18906 (5.50")	Passed (22%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	1625 @ 12' 7 5/8"	4675	Passed (35%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-2411 @ 11' 9 3/8"	5156	Passed (47%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.054 @ 20' 1 1/2"	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.055 @ 20' 1 1/2"	0.234	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (0.2") and TL (2L/240).

· Allowed moment does not reflect the adjustment for the beam stability factor.

• Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Column - SPF	5.50"	5.50"	1.50"	326	2310	2635	Blocking
2 - Column - SPF	5.50"	5.50"	1.50"	387	3099	3486	Blocking
3 - Column - SPF	5.50"	5.50"	1.50"	525	3651	4175	Blocking
4 - Column - SPF	5.50"	5.50"	1.50"	437	2801	3238	Blocking
 Blocking Panels are assumed to carry no load 	s applied dire	ctly above the	m and the ful	l load is appli	ed to the mer	nber being d	esigned.

Lateral Bracing	Bracing Intervals	Comments				
Top Edge (Lu)	20' 2" o/c					
Bottom Edge (Lu)	20' 2" o/c					
Maximum allowable bracing intervals based on applied load						

mum allowable bracing intervals based on applied le

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 20' 1 1/2"	N/A	10.4		
1 - Uniform (PLF)	0 to 20' 1 1/2" (Top)	N/A	72.8	504.8	Linked from: D4, Support 2

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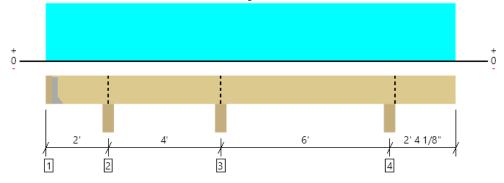
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Overall Length: 14' 4 1/8"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3812 @ 6'	18906 (5.50")	Passed (20%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	1605 @ 6' 10 1/4"	4675	Passed (34%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-2029 @ 6'	5156	Passed (39%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.046 @ 14' 4 1/8"	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.047 @ 14' 4 1/8"	0.234	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (0.2") and TL (2L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

• -253 lbs uplift at support located at 3". Strapping or other restraint may be required.

• Applicable calculations are based on NDS.

	Bearing Length		Loads to Supports (lbs)				
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Hanger on 7 1/2" SPF beam	3.00"	Hanger ¹	1.50"	65	850/-318	915/-253	See note 1
2 - Column - SPF	5.50"	5.50"	1.50"	241	2242/-297	2483/-56	Blocking
3 - Column - SPF	5.50"	5.50"	1.50"	478	3334	3812	Blocking
4 - Column - SPF	5.50"	5.50"	1.50"	471	2851	3323	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

• At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger

• ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments				
Top Edge (Lu)	14' 1" o/c					
Bottom Edge (Lu)	14' 1" o/c					
•Maximum allowable bracing intervals based on applied load.						

 Connector: Simpson Strong-Tie

 Support
 Model
 Seat Length
 Top Fasteners
 Face Fasteners
 Member Fasteners
 Accessories

 1 - Face Mount Hanger
 U66
 2.00"
 N/A
 8-10d
 4-10d

• Refer to manufacturer notes and instructions for proper installation and use of all connectors.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	3" to 14' 4 1/8"	N/A	10.4		
1 - Uniform (PLF)	0 to 14' 4 1/8" (Top)	N/A	77.3	521.3	Linked from: D7, Support 3

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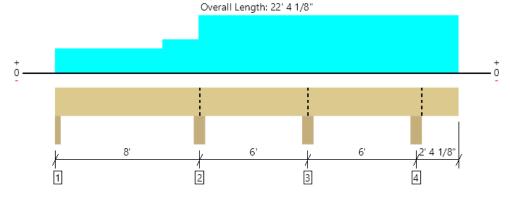
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All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3667 @ 14'	18906 (5.50")	Passed (19%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	1435 @ 14' 10 1/4"	4675	Passed (31%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-2108 @ 14'	5156	Passed (41%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.049 @ 22' 4 1/8"	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.049 @ 22' 4 1/8"	0.234	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (0.2") and TL (2L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

• Applicable calculations are based on NDS.

	Bearing Length		Loads to Supports (lbs)				
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Beam - SPF	2.75"	2.75"	1.50"	125	691/-97	816	None
2 - Column - SPF	5.50"	5.50"	1.50"	421	2679	3100	Blocking
3 - Column - SPF	5.50"	5.50"	1.50"	402	3265	3667	Blocking
4 - Column - SPF	5.50"	5.50"	1.50"	367	2530	2898	Blocking
 Blocking Panels are assumed to carry no load 	s applied dire	ctly above the	m and the ful	l load is appli	ed to the mer	nber being d	lesigned.

Lateral Bracing	Bracing Intervals	Comments		
Top Edge (Lu)	22' 4" o/c			
Bottom Edge (Lu)	22' 4" o/c			

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 22' 4 1/8"	N/A	10.4		
1 - Uniform (PLF)	0 to 5' 11 3/8" (Top)	N/A	28.5	188.3	Linked from: D5, Support 1
2 - Uniform (PLF)	5' 11 3/8" to 7' 11 3/8" (Top)	N/A	36.0	259.5	Linked from: D6, Support 2
3 - Uniform (PLF)	7' 11 3/8" to 22' 4 1/8" (Top)	N/A	58.5	450.8	Linked from: D7, Support 2

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 ForteWEB Software Operator
 Job Notes

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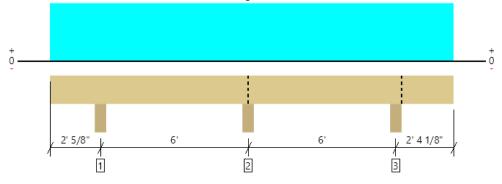
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Deck, D17 1 piece(s) 6 x 8 DF No.1

Overall Length: 16' 4 3/4"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1247 @ 2' 5/8"	12856 (5.50")	Passed (10%)		1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	679 @ 7' 2 3/8"	4675	Passed (15%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-1044 @ 8' 5/8"	5156	Passed (20%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.024 @ 16' 4 3/4"	0.200	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.024 @ 16' 4 3/4"	0.234	Passed (2L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

• Deflection criteria: LL (L/360) and TL (L/240).

• Overhang deflection criteria: LL (0.2") and TL (2L/240).

• Allowed moment does not reflect the adjustment for the beam stability factor.

• Applicable calculations are based on NDS.

	Bearing Length		Loads to Supports (lbs)				
Supports	Total	Available	Required	Dead	Floor Live	Factored	Accessories
1 - Beam - SPF	5.50"	5.50"	1.50"	156	1090	1247	None
2 - Column - SPF	5.50"	5.50"	1.50"	202	1564	1766	Blocking
3 - Column - SPF	5.50"	5.50"	1.50"	169	1174	1343	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	16' 5" o/c	
Bottom Edge (Lu)	16' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

		Tributary Width	Dead (0.90)	Floor Live (1.00)	
Vertical Loads	Location (Side)	Thoulary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 16' 4 3/4"	N/A	10.4		
1 - Uniform (PLF)	0 to 16' 4 3/4" (Top)	N/A	21.8	208.5	Linked from: D7, Support 1

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WOOD COLUMN

6x

Capacity	55 114						
Species:	DF #1						
Size:	6x						
Fc* =	925 psi	Fo	c⊥=	<mark>405</mark> ps	si		<< sill plate is
E =	1.60E+06 psi						Hem-Fir
c' =	0.8						
d =	5.5 in						
KcE =	0.3						
				6x6	6x8	6x10	
le	le	FcE	F'c	Pa	Pa	Pa	
(ft)	(in)	(psi)	(psi)	(lb)	(lb)	(lb)	
Pa (perp)				12251	16149	20604	
/							
8.00	96.00	1576	775	23443	30902	39426	<< crushing governs
8.50	102.00	1396	750	22701	29923	38178	up to a height of
9.00	108.00	1245	724	21897	28864	36826	14'-8" w/ Hem-Fir
9.50	114.00	1117	696	21041	27735	35386	(10'-8" if Doug-Fir)
10.00	120.00	1008	666	20145	26555	33880	
10.50	126.00	915	636	19225	25342	32333	
11.00	132.00	833	605	18296	24117	30770	
11.50	138.00	762	574	17373	22901	29219	
12.00	144.00	700	544	16470	21710	27699	
		, .					

Guard Design

Rail P	200	lb
Section:	HF No. 2 2	κ4
Vn	840	lb
Mn	530	lb-ft
EI	7.00E+06	lb-in2
L max. V	n/a	ft
L max. M	10.60	
L max. El	5.69	ft
L control	5.69	ft
Stanchion		
н	39.75	in
Р	200	lb
Vu	200	lb
Mu	7950	lb-in
Section		x 1-1/2 x 3/16
b	1.5	
t	0.1875	
E	2.90E+07	
fy	50000	
I	0.288	
Z	0.385	in3
Vn	10105	
Mn	11513	
defl	0.50	in

Fasteners

Stanchion to Rim					
No. Screws	4	lag screws			
Arm	4	in			
Withdrawal	994	lb			
d lag	5/8"				
W lag	715	lb/in embed			
Min. embed	1.39	in			
Detail	(4) 5/8"x4"	lag screws			

Rim to Joists		
Arm	5.625	in
Tension	1413	lb
Detail	(2)DTT2Z w	vith 5/8"x4" lag screws @ 48" oc

Blocking to Joists

Arm	<mark>16</mark> in
Zu	497 lb
Cd	1.6
Zn 10d	102 lb
Zn'	163 lb
No. nails	4

Harriott Valentine Engineers Inc.

SECTION 3: LATERAL

SEISMIC LOADS

Per ASCE 7-16 Equivalent Lateral Force Procedure

Occupancy Category	II	Table 1-1
Seismic Design Category	D	Table 11.6-1
Importance Factor	1.00	Table 11.5-1
Site Class	D	Table 20.3-1
Ss	1.43 g	(from SEAOC Design Tool)
S1	<mark>0.50</mark> g	(from SEAOC Design Tool)
Fa	1.20	Table 11.4-1
Fv	1.80	Table 11.4-2
Ct	0.02	Table 12.8-2
х	0.75	Table 12.8-2
hn	12.00 feet	(height to highest level)
Sмs = Fa*Ss	1.7160	Eq. 11.4-1
Sм1 = Fv*S1	0.8961	Eq. 11.4-2
Sds = (2/3)*Sмs	1.1440 g	Eq. 11.4-3
SD1 = (2/3)*Sм1	0.5974 g	Eq. 11.4-4
Period Ta = Ct*hn ^x	0.1289 s	Eq. 12.8-7
To	0.1044 s	per section 11.4.5
Ts	0.5222 s	per section 11.4.5
Sa	1.1440 g	per section 11.4.5
R	1.5	Table 12.2-1
Ωο	1.5	Table 12.2-1
Cd	1.5	Table 12.2-1
Section 12.8 ok?	Yes	Table 12.6-1

Equivalent Lateral Force Procedure (section 12.8)

Cs	0.7627	Eq. 12.8-2
W, weight	6,912 lb	per table below
Q _E	5,272 lb	Eq. 12.8-1

Vertical Force Distribution (section 12.8.3)

k = 1.00

		Floor	Seismic			Wall	Total			(LRFD)	(ASD)
Level	Hx	Area	Dead Ld	Wt.	Length	Wt.	Wt.	WxHx	Cvx	Q_E	0.7Q _E
	(ft)	(ft2)	(psf)	(k)	(ft)	(k)	(k)	(k-ft)	(%)	(k)	(k)
Deck	12.00	768	9	6.9	0	0.0	6.91	82.94	100.00	5.27	3.69
							6.91	82.94	100.00	5.27	3.69

Knee Brace Design

Garcia Residence

0.7E Deck	3690	lb
Deck Area	768	ft2
Deck Height	12	ft

North

North-South loads bear on house

Trib Area	172	ft2
Ratio	0.22	
0.7E	826	lb
No. Braces	4	braces each direction (compression only)
Force	207	lb / brace
Brace Leg	1.875	ft
Brace Force	1870	lb / brace
L Brace	2.65	ft
Pn 4x4	7656	lb OK
Fasteners	2	7/8" thru-bolts
Z	1590	lb
Cd	1.6	
Ζ'	2544	
Z' tot	5088	ІЬ ОК

Central

East-West loads bear on house

Trib Area Ratio 0.7E	301 0.39 1446	
No. Braces	3	braces each direction (compression only)
Force	482	lb / brace
Brace Leg	1.875	ft
Brace Force	4363	lb / brace
L Brace	2.65	ft
Pn 4x4	7656	lb OK
Fasteners	2	7/8" thru-bolts
Z	1590	lb
Cd	1.6	
Ζ'	2544	
Z' tot	5088	lb OK

South

East-West loads bear on house

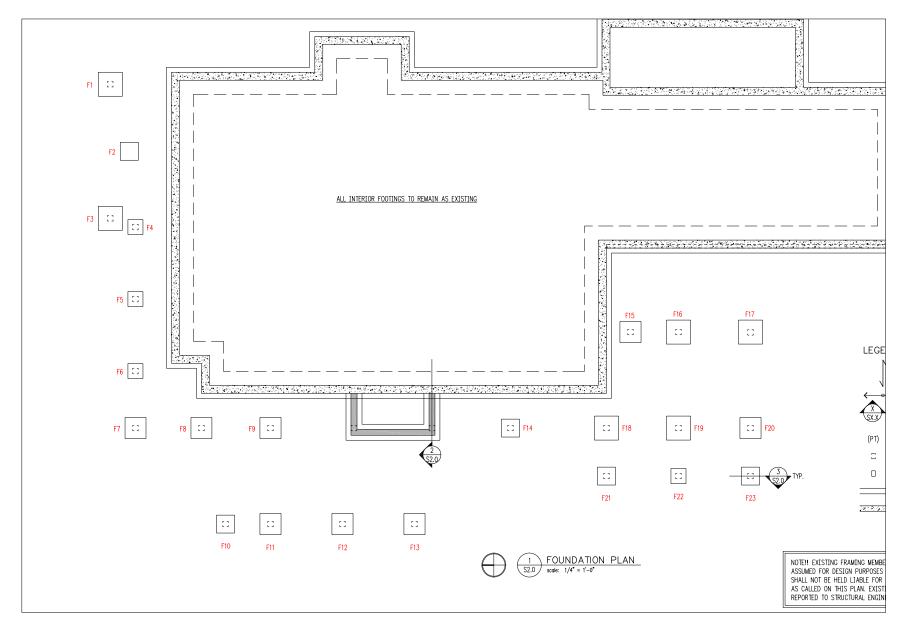
Trib Area Ratio 0.7E	296 0.39 1422	
No. Braces	4	braces each direction (compression only)
Force	356	lb / brace
Brace Leg	1.875	ft
Brace Force	3218	lb / brace
L Brace	2.65	ft
Pn 4x4	7656	lb OK
Fasteners	2	7/8" thru-bolts
Z	1590	lb
Cd	1.6	
Z'	2544	
Z' tot	5088	lb OK

Harriott Valentine Engineers Inc.

SECTION 4: FOUNDATION

Harriott Valentine Engineers Inc.

FOOTING ID



Existing Footings

Garcia

Footing ID	Footing Dim (in.)	Footing Area (ft2)	Force (lb)	Bearing Pressure (psf)
F1	24	4.00	1778	445
F2	18	2.25	0	0
F3	24	4.00	1717	429
F4	15	1.56	631	404
F5	15	1.56	1544	988
F6	15	1.56	1337	856
F7	21	3.06	1286	420
F8	21	3.06	3151	1029
F9	21	3.06	3290	1074
F10	18	2.25	2635	1171
F11	21	3.06	3486	1138
F12	21	3.06	4175	1363
F13	21	3.06	3238	1057
F14	18	2.25	1532	681
F15	21	3.06	2483	811
F16	24	4.00	3812	953
F17	24	4.00	3322	831
F18	24	4.00	3100	775
F19	24	4.00	3667	917
F20	21	3.06	2898	946
F21	18	2.25	1247	554
F22	15	1.56	1766	1130
F23	18	2.25	1343	597

Max. pressure = 1363 psf from footing F12

Window Well Design

Spans N-S between orthogonal walls

L	6.5	ft	
L	78	in	
b min	4.875	in	
b use	6	in	ОК
d	3	in	
detail	#4 @ 12" c	c each way	/
A bar	0.2	in2	
spacing	12	in	
As	0.2	in2 / ft	
fy	60000	psi	
f'c	2500	psi	
а	0.47	in	
Mn	29859	lb-in	
W soil max.	39	lb/in	
W soil max.	471	lb/ft	
ρ soil	120	psf / ft	assumed
h max	3.93	ft	